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Author(s)	Clarke, Rachel; Peyton, Dara; Healy, Mark G.; Fenton, Owen; Cummins, Enda
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2 3 4	A QUANTITATIVE MICROBIAL RISK ASSESSMENT MODEL FOR TOTAL COLIFORMS AND <i>E. COLI</i> IN SURFACE RUNOFF FOLLOWING APPLICATION OF BIOSOLIDS TO GRASSLAND
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7	Rachel Clarke ¹ , Dara Peyton ² , Mark G Healy ² , Owen Fenton ³ and Enda Cummins ¹
8	
9	[Email: rachel.clarke.1@ucdconnect.ie]
10	[enda.cummins@ucd.ie ph. +353 7167476]
11 12	¹ School of Biosystems and Food Engineering, Agriculture and Food Science centre, University College Dublin, Belfield, Dublin 4, Ireland
13	² Civil Engineering, National University of Ireland, Galway, Co. Galway, Rep. of Ireland
14	³ Teagasc, Environment Research Centre, Johnstown Castle, Co. Wexford, Rep. of Ireland
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27 Abstract

The land application of treated municipal sludge ('biosolids') may give rise to surface runoff 28 containing coliforms during episodic rainfall events, which may be potentially harmful to human 29 health if not fully treated in a water treatment plant (WTP). This study used surface runoff water 30 quality data generated from a field-scale study in which three types of biosolids (anaerobically 31 digested (AD), lime stabilised (LS), and thermally dried (TD)) were spread on micro-plots of 32 land and subjected to three rainfall events at time intervals of 24, 48 and 360 hr following 33 application. Under the assumption that this water directly entered abstraction waters for a WTP 34 without any grassed buffer zone being present, and accounting for stream dilution, die-off rate 35 and modelling various performance scenarios within the WTP, the aim of this research was to 36 conduct a human health risk assessment of coliforms (total and faecal), which may be present in 37 38 drinking water after the WTP. Two dose response models for probability of illness were considered for total and faecal coliform exposure incorporating two different exposure scenarios 39 (healthy populations and immuno-compromised populations). The simulated annual risk of 40 illness for healthy populations was below the US EPA and World Health Organisation tolerable 41 level of risk (10⁻⁴ and 10⁻⁶, respectively). However, immuno-compromised populations may still 42 be at risk as levels were greater than the tolerable level of risk for that subpopulation. The 43 sensitivity analysis highlighted the importance of residence time in a stream on the bacterial die-44 off rate. 45

47	Keywords:	biosolids,	coliforms,	risk,	surface	-runoff,	water	treatment
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55 Introduction

The application of treated municipal sewage sludge ("biosolids") to agricultural land as a 56 57 fertiliser can offer an excellent source of nutrients (nitrogen, phosphorus and potassium), increase organic matter and water absorbency, and reduce the possibility of soil erosion. It is also 58 a cost-effective way to dispose of municipal waste and reduce over-reliance on landfill whilst 59 cutting down on tipping fees. However, biosolids can also be non-point source contributors of 60 heavy metals, human pathogens and xenobiotics (Clarke and Cummins, 2014; McCall et al., 61 62 2015; Peyton et al., 2016). Therefore, it is imperative that all biosolids are effectively treated to remove pathogens and contaminants to a "safe level" prior to being used as a land conditioner or 63 fertiliser. 64

More than 10 million tonnes of sewage sludge was produced in the European Union (EU) in 65 2010 (Eurostat, 2014). Although EU policy favours the recycling of resources (COM, 2014), 66 including sludge, national sludge recycling policy varies throughout Europe. In some countries, 67 68 such as the Republic of Ireland, up to 80% of sludge is reused in agriculture (Eurostat, 2014), whereas in other countries, such as Germany, the land application of sludge is prohibited. This is 69 due to the considerable public acceptance issues surrounding the reuse of treated sludge as a 70 fertiliser. The main fear is that the presence of organic and inorganic contaminants in biosolids 71 72 may accumulate in the food chain, or cause the contamination of soil and water (Clarke et al., 2015). The US Environmental Protection Agency (USEPA) Part 503 regulations classify 73 biosolids according to Class A and Class B standard. Class A biosolids contain a faecal coliform 74 density below 1000 most probable number (MPN)/ g of total solids (dry matter, DM), whereas 75 Class B biosolids contain a geometric mean faecal coliform density of less than 2×10^6 MPN / g 76 of total solids (DM) (USEPA 2006). In the USA, the land application of certain types of 77 biosolids requires a class B designation, which must satisfy three different criteria, one of which 78 includes faecal coliforms whose level cannot exceed 2×10^6 MPN/g (Pascual-Benito et al., 79 2015). In the EU, sewage sludge production is regulated by the Sewage Sludge Directive 80 86/287/EC. It does not specify limits for pathogens but instead specifies general land use, 81 harvesting and grazing limits to provide protection against the risk of infection (Sobrados-82 83 Bernardos and Smith, 2012). A revision of the Sewage Sludge Directive (Working Document 3rd Draft) states that "the use of microbial indicators to evaluate the hygienisation of treated 84

sludge is based on fulfilling the limits of *E. coli* to achieve a 99.9% reduction and to less than 1×1 85 10^3 cfu/g dry weight, produce a sludge containing $< 3 \times 10^3$ spores of Clostridium perfringens/g 86 (DM) and absence of salmonella. spp in 50 g (DM)" (EC 2000). Furthermore, the Working 87 Document also states that sludge produced by conventional treatment shall at least achieve a 2 88 log₁₀ reduction of *E. coli* (Mininni et al., 2014). European countries are allowed to include their 89 own parameters in their national regulations (Pascual-Benito et al., 2015). For instance, in France 90 the standards for maximum concentrations of pathogens in biosolids cannot exceed 8 MPN/10 g 91 ¹ DM for *salmonella*, whereas in Finland, the number of *E. coli must be* less than 1000 cfu and 92 Salmonella must not be detected in 25 g of biosolids (Mininni et al., 2014). Meanwhile, in 93 Ireland the standards for maximum concentrations must not exceed 1×10^3 MPN g⁻¹ which is 94 equivalent to Class B biosolids under the USEPA Part 503 regulation (Fehily Timoney and 95 96 Company 1999).

Following land-spreading of biosolids, there are two main scenarios which can lead to human 97 infection. First, pathogens may be transported *via* overland or sub-surface flow to surface and 98 ground waters, and infection may arise via ingestion of contaminated water or accidental 99 ingestion of contaminated recreational water (Jaimeson et al. 2002; Tyrrel and Quinton 2003). 100 Alternatively, it is possible that viable pathogens could be present on the crop surface following 101 biosolids application, or may become internalised within the crop tissue, where they are 102 protected from conventional sanitization (Itoh et al. 1998; Solomon et al. 2002). In this case, a 103 person may become infected if they consume the contaminated produce. Faecal coliform 104 numbers in the stabilised biosolids can be high, up to 10^5 g^{-1} DM (Schwarz et al., 2014). Gerba 105 and Smith (2005) reported general survival times for bacteria in soil to be 2-12 months, whilst 106 Lang et al. (2007) reported survival times of enteric micro-organisms in sludge-amended soil 107 108 varying between 24 hours to 2 years. The disparities in survival rates are difficult to define due to "knowledge gaps" with regards to decay mechanisms and the complex interactions between the 109 110 environment and soil-specific factors that result in the decay of enteric bacteria (Schwarz et al., 2014). Therefore, it is critical to accurately determine the pathogen risk associated with land 111 112 application of sewage sludge to fully understand the potential for environmental loss and consequently, human transmission. 113

Coliforms are bacteria that are always present in the digestive tract of animals including humans, 114 and are found in their waste. They are also found in soil and plant material. Total coliform (TC) 115 bacteria are common in the environment and, with a few exceptions, are generally harmless 116 (USEPA 2013). They are typically used as an indication of other pathogens in drinking water. 117 Faecal coliform bacteria are gram negative, non-spore forming rods that are found in the 118 intestines and faeces of humans and other warm blooded animals. In general, human faecal waste 119 gives rise to the highest risk of waterborne diseases (Odonkor and Ampofo, 2013). The 120 121 predominant faecal coliform is Escherichia coli (USEPA 2006). E. coli is currently recognised by the World Health Organisation (WHO) as the best faecal indicator bacteria for monitoring 122 faecal contamination of drinking water and faecal coliforms are suggested as an acceptable 123 alternative (WHO 2011). E. coli is found in all mammal faeces at concentrations of 10^9 g^{-1} , but 124 does not multiply significantly in the environment (Edberg et al., 2000). High levels of these 125 bacteria indicate the presence of pathogens that cause waterborne diseases (Selvaratnam and 126 Kunberger, 2004). Most coliform bacteria do not cause disease; however, some rare strains of E. 127 *coli*, particularly O157:H7, can cause serious illness. As few as 10 cells can cause serious illness 128 or even death (Liu et al., 2008). Diseases and illness that can be contracted in water with high 129 faecal coliform counts include typhoid fever, hepatitis, ear infections (Oram, 2014), 130 gastroenteritis and, dysentery (Gruber et al., 2014). 131

The WHO recommends that either E. coli or faecal coliforms be used as indicators of faecal 132 133 contamination of water. The WHO guideline value for faecal coliforms (none detected in any 100 ml sample) is reflected in the standards of most OECD members and low-middle income 134 countries (Bain et al., 2014). In their Guidelines for Drinking Water Quality, the WHO have 135 developed a risk classification to prioritise interventions as higher levels of indicator organisms 136 137 are generally indicative of greater levels of faecal contamination. The risk classification is based on the number of indicator organisms in a 100 ml sample which includes <1 'very low risk', 1-138 10 'low risk', 10-100 'medium risk'', > 100 'high risk' or 'very high risk' (WHO, 2011). 139

During wastewater treatment, the sludge component of the waste becomes separated from the water component. As the survival of many microorganisms and viruses in wastewater is linked to the solid fraction of the waste, the numbers of pathogens present in sludge may be much higher than the water component (Straub et al. 1992). Although treatment of municipal sewage sludge

using lime, anaerobic digestion, or temperature, may substantially reduce pathogens, complete 144 sterilisation is difficult to achieve and some pathogens, particularly enteric viruses, may persist. 145 Persistence may be related to factors such as temperature, pH, water content (of treated sludge), 146 and sunlight (Sidhu and Toze, 2009). Similarly, there is often resurgence in pathogen numbers 147 post-treatment, known as the 'regrowth' phenomenon. Taskin et al. (2011) reported a sudden 148 increase in *E. coli* density in anaerobically digested (AD) biosolids immediately after high speed 149 centrifuge dewatering, a phenomena known as 'reactivation' and is separate from growth during 150 151 the storage of dewatered biosolids cake. There are also links to contamination within the centrifuge, reactivation of viable, but non-cultural, organisms, storage conditions post-152 centrifugation (Zaleski et al. 2005), and proliferation of a resistant sub-population due to newly 153 available niche space associated with reduction in biomass and microbial activity (McKinley and 154 155 Vestal 1985). Iranpour and Cox (2006) observed reoccurrence of faecal coliforms in postdigested biosolids from thermophilic anaerobic digestion treatment. The explanations for 156 157 reoccurrence may be linked to 1) incomplete destruction of the faecal coliforms during treatment, 2) contamination from external sources during post-digestion, or 3) a large drop of the post-158 159 digestion biosolids temperature to below the maximum for faecal coliform growth.

160 The European Drinking Water Directive 98/83/EC states that drinking water entering the 161 distribution system should contain zero coliforms and zero *E. coli* in 100 mls (EC 2000). Despite 162 advances in drinking water treatment, the WHO estimates that about 1.1 billion people globally 163 drink unsafe water and the vast majority of diarrhoeal disease (88%) stem from unsafe water, 164 lack of hygiene and sanitation (Ashbolt, 2004).

The objective of this work was to develop a quantitative microbial risk assessment (QMRA) model for coliforms in drinking water assuming the application of biosolids to agricultural land and resulting surface runoff entered abstraction waters for a water treatment plant (WTP).

168 Materials and methods

169 Model development

170 A quantitative drinking water treatment model was developed that was capable of predicting 171 likely human exposure and resulting risk from TC and *E. coli* present in the drinking water 172 without the possibility for attenuation to waters used for WTPs. This represents a pessimistic

scenario as, in reality, biosolids would not be spread to the edge of the field and that grassed 173 buffer zones would be in place. The model was created in Microsoft Excel 2010 with the add-on 174 package @Risk (version 6.0, Palisade Corporation, New York, USA). Uncertainty and 175 variability can be accounted for in the model by means of probability density distributions and 176 are represented in the model's equations by name (e.g. triangular, uniform). Data from peer 177 reviewed scientific literature were incorporated at various steps of the drinking water treatment 178 (i.e. coagulation and flocculation, sedimentation and disinfection). A process-based approach to 179 modelling TC and E. coli fate and human exposure considers total concentration in surface 180 runoff, dilution rate, bacteria die-off rate, drinking water treatment (primary, secondary and 181 tertiary) and human consumption (adult and child). 182

183 Biosolid and soil characterisation

Three types of biosolids were investigated in this study. They were: anaerobically digested 184 biosolids from the UK (AD-UK) and Ireland (AD-IRE), and lime stabilized (LS) and thermally 185 dried (TD) biosolids. With the exception of ADUK, all biosolids originated from the same 186 wastewater treatment plant (WWTP) in Ireland. The ADUK biosolids were sourced from the 187 UK, and were used as part of an EU-funded FP7 project (END-O-SLUDG, 2014). The sludge 188 was collected and land applied to small field plots at the maximum legal application rate in 189 Ireland (Fehily Timoney and Company, 2014) and subjected to three successive rainfall events, 190 191 applied using a rainfall simulator, at time intervals of 24, (RS1) 48 (RS2) and 360 (RS3) hr after application (Peyton et al., 2016). A soil-only control was also included in the experimental 192 design. 193

Three different scenarios (worst case, xxx and yyy) were completed to account for the differences in time and surface runoff volumes. The mean and standard deviation of the surface runoff (Csurface-runoff) of TC and *E. coli*, as measured by Peyton et al. (2016), is shown in Table 1. Runoff results indicated that the AD-UK biosolids had significantly higher concentrations of *E. coli* in the RS1 and RS2 rainfall events, and exceeded the recommended standards of $> 1 \times 10^3$ MPN g⁻¹ (Fehily Timoney and Company 2014). All of the reported Irish biosolids were some 10-fold below the Class A Irish standard (Peyton et al., 2016).

	Total	coliforms	
	Mean and standard devi	ation (n=15) (MPN/ 100	mls)
	RS1	RS2	RS3
AD-UK	171840 ±158962	133516 ± 247832	134860.6 ± 119499
TD	299620 ± 511723.2	615760 ± 629487.1	980600 ± 822835.8
LS	15858 ± 27155.13	628400 ± 820378.8	492000 ± 614760.4
AD-IRE	155220 ± 163536.4	309934.4 ± 503104	197840 ± 190432.9
Control	158220 ± 121426	32850.4 ± 22214.2	470360 ± 506376
		T coli	

Table 1. Mean and standard deviation for total and faecal coliform in surface water. 202

E. coli

Mean and standard deviation (n = 15) (MPN/ 100 mls)

	RS1	RS2	RS3
AD-UK	7055.4 ± 10283.15	4476 ± 5622	210.6 ± 419.6
TD	456 ± 804.3	114 ± 106	44.6 ± 94.23
LS	138.2 ± 21.5	358.2 ± 730.8	39 ± 61
AD-IRE	14.8 ± 21.4	271.6 ± 518.6	199.6 ± 440.7
Control	34.2 ± 47	30.4 ± 51.8	4 ± 8.9

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As a "worst case scenario" it was assumed that surface runoff following biosolid application 205 entered an adjacent stream without any chance of attenuation along the transfer continuum before 206 delivery to the surface water body. This is atypical in terms of grassland management. Schueler 207

et al. (2000) reported on the effectiveness of stream buffers and faecal coliform removal, and
found that grass filter strips were effective in removing up to 70% of faecal coliforms. Similarly,
Coyne et al. (1995) found that grass filter strips removed up to 74% of faecal coliforms from
surface water. However, concentrations of faecal coliforms in surface water still exceeded
minimum concentration standards for primary water.

It was assumed that the runoff effluent in surface-water was then abstracted to a nearby WTP. To 213 214 account for TC and E. coli concentrations in surface water being discharged into the stream, this study used a dilution factor (DF), which is the ratio of concentration in the effluent to 215 concentration in the receiving water after mixing in the receiving water (Colman et al., 2011). 216 This assumes a homogenous distribution of the bacteria in the river and does not account for 217 dispersion or advection. Dilution factors can vary between 1 (dry river bed in summer) up to 218 100,000. The EU Technical Guidance Document on Risk Assessment (2003) states that where 219 there is a lack of specific data, a default dilution value of 10 is recommended for sewage from 220 municipal WTPs when predicting environmental concentrations of contaminants in receiving 221 waters (EC 2003). Therefore, a default dilution factor of 10 was applied to the data to calculate 222 the predicted environmental concentrations in surface water (EQ.1). 223

224
$$PEC_{surface-water} (MPN/100 \text{ mls}) = C_{surface-runoff} / DF Eqn. 1$$

Where $PEC_{surface-water}$ is the concentration of coliforms (TC and *E. coli*) in surface waters receiving wastewater effluent, DF is the dilution factor, and ($C_{surface-runoff}$) (MPN/100 mls) is the concentration in surface water

The first order decay equation often used to describe bacterial die-off is expressed as Chick's 228 229 Law, and is used to describe the survival (die-off rate) of TC and E. coli in soil, manure, streams and groundwater over time (Benham et al., 2006). Die-off is a function of temperature, nutrient 230 levels, competing bacteria and solar radiation (Hrudey, 2004). The rate of bacterial "die-off" is 231 greater in summer than winter due to higher temperatures and increased UV light (Murphy et al., 232 233 2015). Wilkinson et al. (1995) reported enhanced coliform concentrations in streams during high and rising flows following storm events. The die-off rate in stream (D-off) was calculated 234 235 according to Eq. 2:

$$Nt = N_0 e^{(-kt)}$$
 Eqn. 2

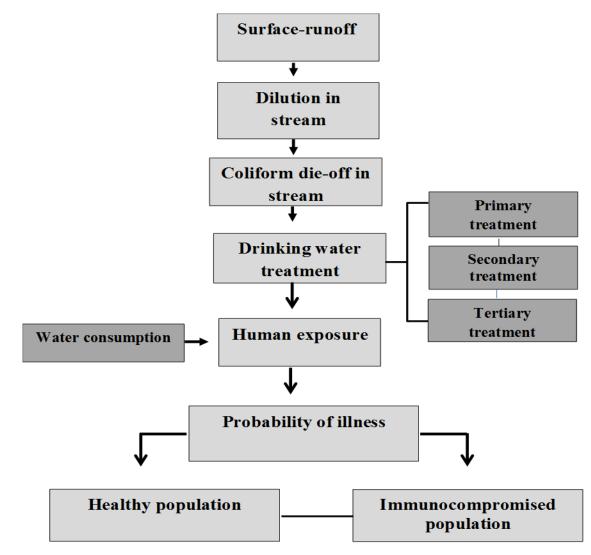
Where Nt is the number of coliforms at time t in surface-water (MPN/100 mls), N₀ is the original number of coliforms following dilution in surface-water (PEC_{surface-water}) (MPN/100 mls), k is the first order inactivation constant (d^{-1}), and t is the time in the stream (d^{-1}).

The k value was incorporated according to Schueler (2000), using a uniform distribution (values min 0.7 and max $1.5 d^{-1}$). k values in this range mean that about 90% of the bacteria present will disappear from the water within 2 to 5 days. Therefore, it was assumed that water was abstracted for drinking water treatment from the stream to a nearby WTP between 0 and 5 days. To account

for uncertainty, the time in stream "t" was fitted with a uniform distribution (min 0, max 5 day⁻¹).

245 Drinking water treatment processes

246 There are typically three stages to drinking water treatment (primary, secondary and tertiary) (Figure 1). The three stages of drinking water treatment that were used were based on the Irish 247 Environmental Protection Agency's (EPA) best practice guidelines for drinking water treatment 248 manuals (EPA 1995, 2002, 2011). It is assumed that operations within the drinking water 249 250 treatment process are running efficiently or stable (C-opt). However, to account for inefficiencies in treatment operations, a sub-optimal (CS-opt) and failure (C-fail) option were incorporated into 251 252 the model. Poor operation of filters and inadequate disinfection may pose a risk to human health. In recent times, many WTPs have become automated. 253



255

Figure 1. Flow diagram of the quantitative risk assessment drinking water model for coliformsin biosolids applied to grassland.

The first stage (primary treatment) considers the screening, storage, pre-conditioning and pre-259 chlorination of the water. In the current study, primary treatment was assumed to have a 260 negligible impact on coliform removal. Secondary treatment involves the coagulation, 261 flocculation, sedimentation and filtration of the influent. Coagulation/flocculation, sedimentation 262 and filtration remove particles, including microorganisms (bacteria, viruses and protozoa) (WHO 263 2011). The commonest types of coagulants used are aluminium-based (e.g. aluminium sulphate 264 (alum) or polyaluminium chloride (PAC)). Both aluminium and ferric salts, either in monomer or 265 polymeric forms, have been reported as effective coagulants in treating wastewater (Kang et al., 266

- 267 2003; Pang et al., 2009). When properly performed, coagulation, flocculation and sedimentation 268 can result in 1-2 log removal of bacteria, viruses and protozoa (WHO 2004). In accordance with 269 the Irish EPA's guidance manual (Ireland EPA 2002), the coagulant considered was aluminium 270 sulphate (Al₂ (SO₄)₃ (referred to as alum) for both TC and *E. coli*.
- 271 As faecal coliforms are the indicator organism for *E. coli*, reductions in *E. coli* counts were used to account for variability and uncertainty in the data. Pritchard et al. (2010) compared the 272 273 efficacy of alum sulphate to more natural coagulants and reported E. coli reductions of 89% using 30-50 mg L⁻¹ of alum sulphate. Bulson et al. (1984) reported removal rates of *E. coli* of 274 99.99% following a dose of 15 mg L^{-1} of alum sulphate. A study conducted by Sarpong et al. 275 (2010) showed that total coliform counts were reduced by 95% using a 5 ml dose of alum 276 277 sulphate. Similarly, Bergamasco et al. (2011) reported a 99% reduction in total coliforms using a 15 ml dose of alum sulphate. Thus, a uniform distribution was used to model coagulation, 278 flocculation and sedimentation incorporating a decimal reduction to account for variability and 279 uncertainty in the data (min 0.89, max 0.99). 280
- As a "worst case scenario", the model assumes a 90% probability of coagulation and flocculation occurring at an optimum stable run (Copt) and 5% probability for both sub-optimal (CS-opt) and failure (C-fail) (Table 2). When operating optimally, the model assumes a removal rate (uniform distribution min 0.89, max 0.99) (Table 3). When operating sub-optimally, the model assumes a removal of 50% of the optimal removal rate, and zero removal during failure events. It was assumed that aluminium sulphate was applied at an optimum dose of approximately 10 mg L^{-1} .
- 287

Stage	Symbol	Description	Model /distribution	Units
		Effluent (Surface-ru	noff)	
	$C_{surface-runoff}$	Initial concentration in surface	Lognormal	MPN/ 100mls
		runoff		
			(based on Table 1)	
Dilution	DF	Dilution in stream	Dilution factor (10)	-
	PEC _{surface-water}	Concentration of coliforms in	C _{surfacerunoff} / DF	MPN/100 mls
		surface-water following dilution		

Table 2. Model inputs and distributions

Stage	Symbol	Description	Model /distribution	Units
Die-off	K	First order inactivation constant	Uniform	d ⁻¹
	t	Time in stream	Ûniform	d^{-1}
			(min 0, max 5)	u
	D-off	Die-off rate in stream	$N = N_0 [exp(-kt)]$	MPN/ 100mls
		Secondary treatn	nent	
	C-opt	Coagulation/Flocculation and sedimentation optimum	0.90	Probability
	CS-opt	Coagulation/Flocculation and sedimentation sub-optimum	0.05	Probability
	C-fail	Coagulation/Flocculation and sedimentation fail	0.05	Probability
	Cr	Coagulation/Flocculation and	Uniform	Decimal
		sedimentation reduction	(min 0.89, max 0.99)	reduction
	F-opt	Filter optimum run	0.9	Probability
	FS-opt	Filter sub-optimum run	0.1	Probability
	Frd	Filter reduction (rapid sand)	Uniform (min 0.74, max 0.99)	Decimal reduction
		Tertiary treatme	ent	
	D	Disinfection	Uniform	Decimal
			(min 0.97, max 0.99)	reduction
Output	Pstt	Post-secondary and tertiary treatment	Pstt =D-off × (1-Cr) × (1- Frd) × (1-D)	MPN/ 100mls
		Human exposu	· · · · ·	
Consumption	TWi	Tap water intake (adult)	Lognormal	L d ⁻¹
			(mean 0.564,	
Output	Vcc	Viable coliforms/ <i>E. coli</i> consumed	Pstt × Twi	MPN/ d ⁻¹
		Dose response		
Output	I(H)	Probability of illness (healthy)	1-EXP (- 0.0000005 × Vcc)	-
Output	I (Ic)	Probability of illness (immunocompromised)	1-EXP (- 0.01 × Vcc)	-

The filtration process is the last treatment stages that can physically remove contaminants before 291 292 disinfection. One of the most popular filtration processes used in Ireland is the rapid gravity sand process (Ireland EPA 1995). A study by Li et al. (2012) showed that direct rapid sand removal 293 294 can remove 0.6 - 1.5 log-units of total faecal coliform, depending on the loading rate and grain size distribution. Mwabi et al. (2012) demonstrated that designing and building a bio-sand 295 filtration system was effective in removing 2 - 4 log_{10} of coliform bacteria. Koivunen et al. 296 (2003) showed that tertiary treatment by the rapid sand filtration process found, on average, a 297 97% reduction of faecal coliforms and total coliforms in four conventional wastewater treatment 298 plants in Helsinki, Finland. In keeping with the Irish EPA's filtration manual guidelines, rapid 299 gravity filtration was considered in the model. Filtration can be stable or unstable due to 300 optimum, sub-optimum or failure of the coagulation/flocculation process. As a "worst case 301 scenario" the model assumes a 90% probability of filtration operating at an optimum stable run 302 (Fopt) and 10% probability for sub-optimal run (FS-opt). To model rapid sand filtration under 303 optimum conditions and to account for uncertainty and variability in the data, a decimal 304 reduction uniform distribution was assigned (min 0.74 max 0.99) (Table 2). When operating sub-305 optimally, the model assumes a removal of 50% of the optimal removal rate. 306

307 **Disinfection**

Disinfection is the process by which an organism's viability/infectivity is destroyed with a 308 specific percentage of the population dying over some time frame defined as a rate (Betancourt 309 and Rose, 2004). Worldwide, chlorine is the most commonly used disinfection in drinking water 310 treatment, although other alternatives are being increasingly introduced such as ozonation, 311 ultraviolet irradiation, ultrasonic vibration, ultra-filtration, silver, bromide and iodine, membrane 312 filtration and granular activated carbon (GAC). Chlorine is added to provide a disinfectant 313 residual to preserve the water in distribution, where the chlorine is in contact with the water for a 314 longer period of time compared to the pre-chlorination process in primary treatment (Irish EPA, 315 2011). The principal factors that influence disinfection efficiency are the disinfection 316 concentration, contact time, temperature and pH (depending upon the disinfection) (Cotruvo et 317 al., 2013). Chlorination has been found to remove E. coli between 97-99% (O' Connor and O' 318 Connor 2001). However, a report by Igunnnugberni et al. (2009) showed that water storage post 319

chlorination significantly reduced survival of *E. coli* and that the presence of *E. coli* following chlorination could undermine the effectiveness of chlorination. To account for uncertainty in the data, a uniform distribution (minimum 0.97, maximum 0.99) was assigned to model the inactivation attributed to the disinfection process.

Removal of coliforms and bacteria (TC and *E. coli*) was quantified in terms of a decimal reduction. The concentration of coliforms remaining after secondary and tertiary treatment in a WTP was calculated by multiplying the level present post primary treatment by the decimal reduction due to coagulation/ flocculation, sedimentation, filtration and disinfection. The equation is:

329
$$P_{STT} = D \text{-off} \times (1 \text{-Cr}) \times (1 \text{-Frd}) \times (1 \text{-D})$$
 Eqn.3

Where: P_{STT} is the coliform concentration post-secondary and tertiary treatment (MPN/100 mls), Cr is decimal reduction due to coagulation /flocculation and sedimentation, Frd is decimal reduction due to filtration, and D is the decimal reduction due to disinfection.

333 Human Exposure

Water consumption in Ireland for adults was modelled using a lognormal distribution with a mean and standard deviation value of 0.564 ± 0.617 L d⁻¹ according to a survey on adult consumption patterns conducted by the Irish Universities Nutrition Alliance (IUNA) which was based on 1274 consumers. The same survey was used to model variation in adult body weight (males and females) and a normal distribution with a mean and standard deviation value of 78 ± 16.5 kg was used (IUNA, 2011).

340 Dose response model

In order to assess the risk to human health from coliforms and *E. coli* associated with water consumption, the potential exposure to the organism(s) in the daily drinking water intake was estimated. Exponential models are widely used in microbial risk assessment (Teunis et al., 2004). The exponential model assumes that pathogen-host interactions can describe the pathogen-host survival probability by a discreet value (Haas et al., 2000). Two dose response models were considered for TC and *E. coli* exposure incorporating two different exposure scenarios (healthy populations and immuno-compromised populations). Immuno-compromised individuals include

patients on active anti-cancer drugs, HIV/AIDS and other chemotherapies. Allen et al. (2013) 348 defines an immuno-compromised individual as having a haematology profile showing abnormal 349 values for gamma globulins, white blood cells, red blood cells and liver function. The dose 350 response model estimated the probability of illness resulting from a certain level of exposure. An 351 exponential dose-response model was used for probability of illness, integrating an "r" value of 352 0.01 for immuno-compromised populations (I(Ic)) and an "r" value of 0.0000005 for healthy 353 population (I(H)) as proposed by Gale (2005). As a "worst case scenario", the illness model was 354 parameterized with the assumption that the virulence of the pathogen is similar to E. coli 355 O157:H7. The E. coli O157:H7 strain is a particular serotype of the group referred to as 356 verocytotoxigenic E. coli (VTEC). VTECs produce verotoxins or shiga-like toxins that are 357 closely related to the toxin produced by Shigella dysenteriea (Cassin et al., 1998). The USEPA 358 have proposed an acceptable benchmark of 10⁻⁴ annual infection/illness probability per person per 359 year for Shigella (Grant et al., 2012). The WHO use the metric DALY (disability-adjusted life 360 year) to estimate severity and duration of a disease. The 10⁻⁶ DALY tolerable burden of disease 361 may be considered unrealistic and there have been proposals to introduce a less stringent burden 362 of risk such as the upper limit for excess lifetime risk of cancer of 10^{-5} or a 10^{-4} limit in line with 363 the USEPA limit (WHO 2011). Crockett et al. (1996) reported that ingestion of only 10-100 364 Shigella cells can lead to infection. The probability of illness per day can be expressed by: 365

366

$$P_i = 1 - \exp(-d \times r)$$
 Eqn. 4

 $P_{i(365)} = 1 - (1 - P)^{365}$

Where P_i is the probability of illness (d⁻¹), d is the dose and 'r' represents an exponential parameter. The annual individual risk is calculated as:

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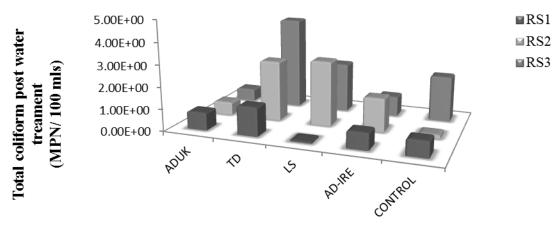
371 Sensitivity analysis

A sensitivity analysis, based on rank order correlation, was carried out to assess how the model's predictions are dependent on variability and uncertainty in the model input parameters. Sensitivity analysis assesses how the model predictions are dependent on variability and uncertainty in the model's inputs. Monte Carlo simulation performs risk analysis by building models of possible results by substituting a range of values—a probability distribution—for any
factor that has inherent uncertainty or variability (Kavcar et al., 2009). It then iterates the results
using a different set of random values from the probability functions. Ten thousand iterations
were performed for each simulation.

380 **Results**

Figures 2 and 3 give the predicted results of TC and E. coli remaining following drinking water 381 treatment under the scenarios considered. The model produced several output distributions (TC 382 and E. coli concentration in effluent post WTP, viable coliforms consumed, and probability of 383 illness) that can be used to compare the concentration of coliforms that were detected in surface 384 runoff and their potential risk to human health. The model predicted that surface runoff arising 385 from the land spreading of TD biosolids and ADUK biosolids produced the highest 386 concentrations of TC and E. coli, respectively, in drinking water. The modelled mean TC and E. 387 *coli* concentration in drinking water was highest when the surface runoff concentrations from the 388 TD and ADUK, respectively, biosolids at each rainfall simulation time (24, 48 and 360 hr) were 389 used as input into the model (mean concentration values 1.3, 2.7 and 4.2 MPN/100 mls for TC 390 and TD biosolid treatment (Figure 2), and 7.3×10^{-2} , 4.7×10^{-2} and 2.4×10^{-3} for *E. coli* and 391 ADUK biosolid treatment (Figure 3). 392

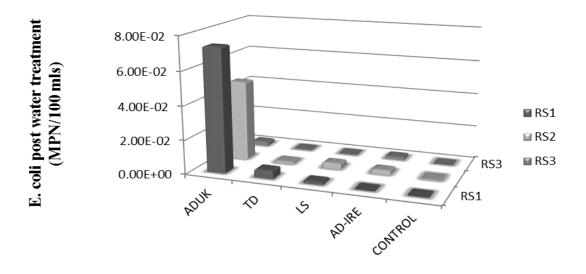
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Biosolid treatments

Figure 2. Simulated mean total coliforms remaining following drinking water treatment

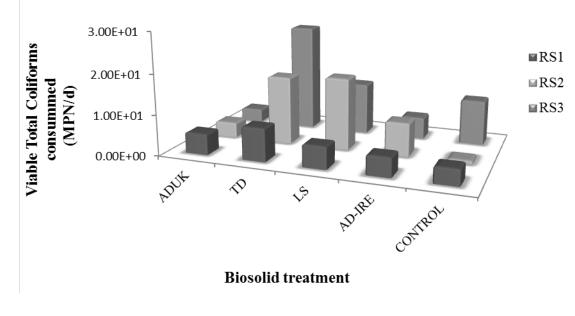
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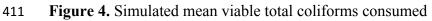


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Figure 3. Simulated mean *E. coli* remaining following drinking water treatment

The EU states that there should be 0 in 100 mls of coliform bacteria and E. coli following 400 drinking water treatment. The results for mean human exposure via drinking water consumption 401 show that for TC the greatest viable coliforms consumed was for the biosolid TD and LS 402 combining rainfall simulation times of 48 and 360 hr (RS2 and RS3), respectively (Figure 4) 403 (mean viable total coliform values 16.83 and 26.75 MPN d⁻¹, respectively) for TD (mean viable 404 total coliform values 17.74 and 12.82 MPN d^{-1} , respectively) for LS biosolids. The results for E. 405 coli show that the greatest viable coliforms consumed was for ADUK biosolids and rainfall 406 simulation times of 24 and 48hrs (RS1 and RS2) (Figure 5) mean viable faecal coliforms 407 consumed values 5.20×10^{-1} and 2.34×10^{-1} MPN d⁻¹, respectively. 408





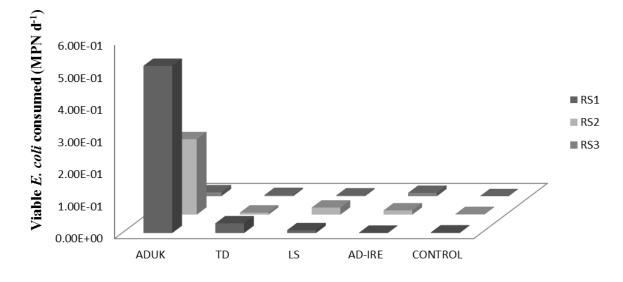


Figure 5. Simulated mean viable *E. coli* consumed

The results for probability of illness (healthy and immuno-compromised) are displayed in Table 416 3. For each scenario (healthy and immuno-compromised), the risk assessment model produced a 417 simulated probability of illness per day and per year. Compared to the healthy population, the 418 immuno-compromised population are more at risk of illness with mean annual values for TC and 419 immuno-compromised (9.92 \times 10⁻¹) and LS biosolids treatment (RS1), (7.24 \times 10⁻¹ and 7.87 \times 420 10⁻¹) for the TD biosolids treatment incorporating the RS2 and RS3 time frames, compared to 421 mean annual values for TC and healthy population for the same biosolid treatments and time 422 frames (1.01×10^{-3}) incorporating the LS biosolids treatment and RS1 time frame, mean annual 423 values for healthy population and TD biosolids treatment $(2.77 \times 10^{-3} \text{ and } 4.27 \times 10^{-3})$ 424 respectively), incorporating the RS2 and RS3 time frames. The mean annual values for E. coli 425 and immuno-compromised populations show that the ADUK biosolids and the RS1 and RS2 426 time frames had the greatest probability of risk (values 2.1×10^{-1} and 1.7×10^{-1} , respectively). 427 This is comparable to the healthy population for the same biosolid treatment and time frames 428 (mean annual values of 7.0×10^{-5} and 4.6×10^{-5} , respectively). 429

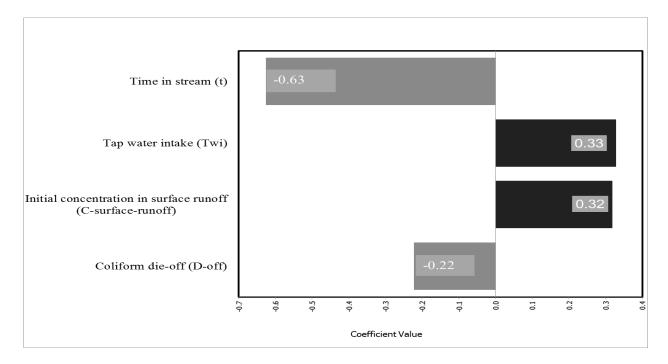
				Probability of	illness			
		Healthy	population		Immuno	-compromised	l population	
Biosolid treatment	(d ⁻¹)	(1	v r ⁻¹)	(d ⁻¹)		(yr	⁻¹)
	TC	E. coli	TC	E. coli	TC	E. coli	TC	E. coli
				RS1				
ADUK	2.57E-06	1.71E-07	8.90E-04	7.0E-05	2.81E-02	3.68E-03	5.62E-01	2.1E-01
TD	4.09E-06	1.46E-08	1.35E-03	4.2E-06	3.67E-02	2.86E-04	5.85E-01	4.1E-02
LS	2.76E-06	5.29E-09	1.01E-03	1.3E-06	5.22E-02	1.03E-04	9.92E-01	1.9E-02
AD-IRE	2.38E-06	3.94E-10	7.89E-04	1.4E-07	2.44E-02	7.87E-06	5.38E-01	2.5E-03
CONTROL	2.00E-06	9.51E-10	7.18E-04	3.9E-07	2.60E-02	1.90E-05	5.62E-01	6.2E-03
				RS2				
ADUK	1.86E-06	1.23E-07	6.34E-04	4.6E-05	2.05E-02	2.1E-03	4.65E-01	1.7E-01
TD	8.41E-06	3.46E-09	2.77E-03	1.0E-06	6.73E-02	6.9E-05	7.24E-01	1.5E-02
LS	8.86E-06	9.26E-09	2.71E-03	3.4E-06	6.32E-02	1.8E-04	7.10E-01	3.5E-02
AD-IRE	4.11E-06	6.35E-09	1.41E-03	3.0E-06	3.87E-02	1.3E-04	5.94E-01	2.9E-02
CONTROL	3.91E-07	8.69E-10	1.42E-04	3.2E-07	6.83E-03	1.7E-05	3.43E-01	5.2E-03
				RS3				
ADUK	1.66E-06	6.1E-09	5.99E-04	2.6E-06	2.30E-02	1.2E-04	5.31E-01	2.3E-02
TD	1.34E-05	1.2E-09	4.27E-03	1.2E-06	9.31E-02	2.4E-05	7.87E-01	6.7E-03
LS	6.41E-06	1.2E-09	2.20E-03	3.9E-07	5.47E-02	2.3E-05	6.82E-01	6.4E-03
AD-IRE	2.70E-06	5.1E-09	9.38E-04	2.3E-06	2.97E-02	1.0E-04	5.80E-01	2.3E-02
CONTROL	5.56E-06	1.1E-10	1.93E-03	4.6E-08	5.35E-02	2.2E-06	6.89E-01	8.6E-04

Table 3. Probability of illness for healthy and immuno-compromised populations

Sensitivity analysis was performed to investigate how variability of the outputs can be 1 apportioned quantitatively to different sources of variability in the inputs. The analysis indicated 2 that the LS and TD biosolids produced the highest concentration post WTP of TC, and ADUK 3 produced the highest concentration of *E. coli*, in drinking water, therefore, a sensitivity analysis 4 was conducted for the annual probability of illness for both biosolid treatments. Results for TC 5 and E. coli show that the parameter of importance that affected the variance in model predictions 6 was time in the stream (correlation coefficient -0.63 and -0.57, respectively) (Figures 6 and 7). 7 8 This highlights the importance of residence time of bacteria in stream. The longer the bacteria are in the stream, the more likely the bacteria are subject to factors such as temperature, pH and 9 photolysis, which may in-turn influence the growth or die-off rate of bacteria in a stream. The 10 other parameters of importance were the tap water intake and initial concentrations in surface 11 12 runoff (correlation coefficients 0.33 and 0.31, respectively, for Twi and 0.32 and 0.33, respectively, for C-surface-runoff). The die-off rate in the stream (-022 for TC and -0.20 for E. 13 14 *coli*) was also of importance. The die-off rate is related to the residence time in the stream and is associated with sub-optimum conditions in the stream that influence bacterial growth. 15

16

17



- 1 Figure 6. Sensitivity analysis for TC annual probability of illness and TD biosolid treatment
- 2
- 3

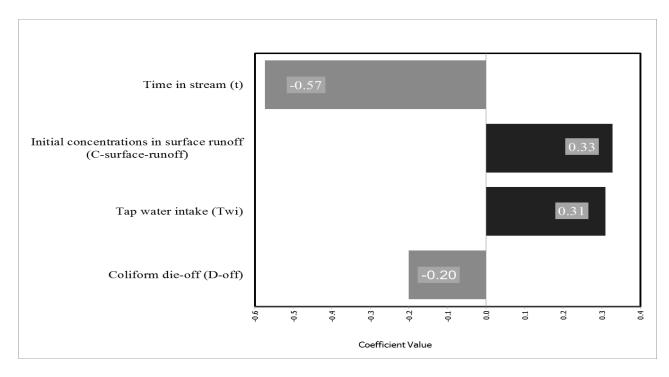


Figure 7. Sensitivity analysis for *E. coli* annual probability of illness and ADUK biosolid
treatment

7 Discussion

4

Concentrations of TC and E. coli in surface runoff following the spreading of biosolids on 8 9 grassland were quantitatively assessed to study their fate in drinking water treatment and subsequent consumption and human health effects. Initial concentrations of E. coli in surface 10 runoff were above the recommended standards of $> 1 \times 10^3$ MPN g⁻¹ and were equivalent to class 11 B microbial matter under the USEPA Part 503 regulations. Surface runoff is distinguished from 12 other types of runoff in that it does not pass through the soil. Therefore, typical soil-pathogen 13 reactions (desiccation, photolysis, temperature and nutrients) may be by-passed depending on the 14 rate off rainfall. Concentrations of TC and E. coli in surface runoff in this study are comparable 15 to concentrations reported by Le Chevalier et al. (1991) and Schreiber et al. (2015). All TC and 16

E. coli concentrations had decreased by the third rainfall event (RS3; 360 hr) due to desiccation
 of the pathogens in soil following the application of the biosolids.

3 The mean concentration of TCs after the WTP showed that the TD biosolids (RS2 and RS3) and LS biosolids (RS2 and RS3) with the highest concentration of TC. This was attributed to initial 4 5 concentrations of TCs in the influent and the time in stream combined with the removal rates 6 associated with secondary treatment (e.g. coagulation/flocculation and sedimentation and 7 filtration). Thermal drying is recognised as more effective in pathogen removal than mesophilic digestion and can achieve the time-temperature requirement for Class A biosolids (Iranpour and 8 9 Cox, 2006). However, regrowth of pathogens can occur in thermally dried biosolids (Zaleski et al. 2005). Lloret et al. (2013) showed that the reduction in sludge retention time may be 10 responsible for presence of coliforms post treatment. Lloret et al. (2013) reported that a 11 minimum time of more than 10 days under thermophilic conditions is required to achieve 12 appropriate sanitation of sludge. Similarly, Iranpour and Cox (2006) reported the presence of 13 faecal coliforms after thermal drying, and attributed the reason to be the relatively short slude 14 retention time of about 10 days. 15

The mean concentration of *E. coli* post drinking water show that the ADUK biosolids had the greatest concentration of *E. coli* for RS1 and RS2 only. This was also attributed to the initial concentration and the time in the stream of *E. coli* in the influent and associated drinking water treatment removal rates. Although initial concentrations of TC and *E. coli* in surface water were high, the effect of drinking water treatment significantly reduced overall TC and *E. coli* concentrations with a 99.9% reduction across all treatments and time frames.

22 The mean viable consumption of TC and E. coli in drinking water showed the same trends as mean TC and E. coli concentrations post drinking water treatment. Safe drinking water is a 23 human right and in developed countries it has become an "entitlement". Water consumers rely on 24 25 the efficacy of drinking water treatment to produce a product that is pathogen free, odourless and clear. However, indicator bacteria are known to regrow in finished drinking water. This was 26 highlighted in a report by Le Chevalier et al. (1991). The authors reported various factors 27 attributed to the occurrence of coliforms in drinking water including disinfectant residuals, 28 filtration and temperature. Bacterial growth can occur on any surface that is constantly wet, so 29 the internal surface of water distribution pipes is normally coated with a biofilm (Gray, 2010). 30

Although the concentrations of coliforms post drinking water treatment in this study were
 significantly reduced, inefficiencies in drinking water treatment due to operational defects that
 promote the regrowth of coliforms and other pathogens can be a cause of concern for drinking
 water management.

5 Ideally water intended for human consumption should be pathogen free. However, in practice, this is an unachievable goal. A consequence of variable human susceptibility to pathogens is that 6 7 exposure to drinking water of a particular quality may lead to health problems in different populations (WHO 2011), particularly the very young and immuno-compromised. Enteric 8 pathogens are among the many agents that take advantage of the impaired or destroyed immune 9 system; therefore, sensitive populations are considerably more vulnerable and may need special 10 protection from waterborne microorganisms (Gerba et al., 1996). As E. coli is used as an 11 indicator that faecal matter is present, it may indicate the presence of pathogens that cause 12 waterborne diseases. The risk of illness for healthy populations was deemed negligible based on 13 the tolerable risk guidelines set by the USEPA and the WHO for Shigella. However, based on the 14 15 same guidelines, immuno-compromised populations may be at risk. Individuals who are truly immuno-compromised would follow medical advice regarding food and water intake, thus 16 reducing the risk of illness. 17

18

The Sewage Directive has yet to address the bacteriological quality of treated water. The current European legislation requires that the sludge be subjected to a process of stabilisation before land application. With future demography increases and growing demand for water, the use of reclaimed water will rise; therefore efforts to assess the treatment efficacy are vital.

23

24 Conclusions

Application of biosolids on grassland and subsequent simulated rainfall over three time frames resulted in TC and *E. coli* counts in surface runoff. The concentrations of *E. coli* exceeded the recommended standards being some 10-fold below the Class A Irish standard. This prompted the need to investigate human exposure. Further analysis which included simulated dilution and dieoff rate in a stream, drinking water treatment, and human exposure following consumption of the treated water resulted in a very low probability of illness based on the USEPA and the WHO

threshold of acceptable risk $(10^{-4} \text{ and } 10^{-6}, \text{ respectively})$ for healthy populations. However, the 1 risk of illness for immuno-compromised populations exceeded the thresholds of acceptable risk 2 3 by a factor of 3 for TC and a factor between 1-3 for E. coli. It is noted in such cases, susceptible populations would be subject to medical advice regarding food and water intake, thus reducing 4 the risk of illness. The sensitivity analysis identified that the time in stream is an important 5 parameter as the longer the bacteria are in the water and being exposed to ultraviolet light, 6 varying temperature and pH, the greater the influence on bacterial growth. The risk assessment 7 8 model developed in this study may be of importance to local authorities or regulatory agencies to evaluate the likely risk of E. coli entering potable water following biosolid application on 9 agricultural land. As this study only focused on coliforms, future studies are needed in order to 10 assess other compounds of concern e.g. pharmaceutical contaminants that may be present in 11 12 biosolids.

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3	Programme (2007-2013).
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